## Category: Large Deformation Analysis, Isotropic Linear Elasticity, Member

## Verification Example: 0043 - Cantilever Bend to Form a Circle

## 0043 - Cantilever Bend to Form a Circle

## Description

Determine the bending moment $M$, which acts at the free end of the cantilever and which bends the member to a circular shape. Neglecting beam's self weight, assuming the large deformation theory and loading the cantilever with this particular $M$, check the maximum deflections $u_{X, \max }$ and $u_{Z, \text { max }}$.

| Material | Steel | Modulus of Elasticity | E | 210.000 | GPa |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Shear <br> Modulus | G | 81.000 | GPa |
| Geometry | Beam | Length | L | 4000.000 | mm |
|  |  | Diameter | $d$ | 42.400 | mm |
|  |  | Wall <br> Thickness | $t$ | 4.000 | mm |



Figure 1: Problem sketch

## Analytical Solution

The second moment of inertia around $y$ axis $I_{y}$ equals to (see Figure 1):

$$
\begin{equation*}
I_{y}=\frac{\pi\left[d^{4}-(d-2 t)^{4}\right]}{64} \approx 89908.5 \mathrm{~mm}^{4} \tag{43-1}
\end{equation*}
$$

A beam in the large deformation analysis is described by the nonlinear differential equation

$$
\begin{equation*}
\kappa(x)=\frac{u_{z}^{\prime \prime}(x)}{\left[1+\left(u_{z}^{\prime}(x)\right)^{2}\right]^{\frac{3}{2}}}=-\frac{M}{E I_{y}} \tag{43-2}
\end{equation*}
$$

which is an equation difficult to solve in general. However, the term on the right-hand side is constant and consequently the left-hand side, which is nothing else then the beam curvature $\kappa$, is constant also. The only curve which has constant curvature is a circle, therefore, the solution to this problem is a circle arc of radius $R$. We get

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$$
\begin{align*}
& u_{x, \max }=R \sin \alpha-L \\
& u_{z, \max }=R(1-\cos \alpha)
\end{align*}
$$

where the radius of the circular arc equals to

$$
\begin{equation*}
R=\left|\frac{1}{\kappa(x)}\right|=\frac{E I_{y}}{M} \tag{43-5}
\end{equation*}
$$

The angle of the circular arc equals to $\alpha=\frac{L}{R}$. In our case $\alpha=2 \pi$, which yields

$$
\begin{equation*}
R=\frac{L}{2 \pi} \approx 636.620 \mathrm{~mm} \tag{43-6}
\end{equation*}
$$

The equations (43-5) and (43-6) yield the required loading moment

$$
\begin{equation*}
M=2 \pi \frac{E I_{y}}{L} \approx 29657.585 \mathrm{Nm} \tag{43-7}
\end{equation*}
$$

Moreover, equations (43-3) and (43-4) yield the unknown maximum displacements

$$
\begin{align*}
& u_{X, \max }=-L=-4000.0 \mathrm{~mm}  \tag{43-8}\\
& u_{z, \max }=-2 R \approx-1273.2 \mathrm{~mm} \tag{43-9}
\end{align*}
$$

## RFEM 5 and RSTAB 8 Settings

- Modeled in version RFEM 5.05.0030 and RSTAB 8.05.0030
- The element size is $I_{\mathrm{FE}}=0.004 \mathrm{~m}$
- The number of increments is 1
- Isotropic linear elastic material model is used
- Member division for large deformation or post-critical analysis is activated


## Results

| Structure File | Program |
| :---: | :---: |
| 0043.01 | RFEM 5 |
| 0043.02 | RSTAB 8 |

Good agreement of the numerical results with the analytical solution was achieved:

| Displacement | Analytical <br> Solution | RFEM 5 |  | RSTAB 8 |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $[\mathrm{mm}]$ | $[\mathrm{mm}]$ | Ratio [-] | $[\mathrm{mm}]$ | Ratio [-] |
| $u_{X, \max }$ | -4000.0 | -3998.5 | 1.000 | -4000.0 | 1.000 |
| $u_{Z, \max }$ | -1273.2 | -1273.1 | 1.000 | -1273.2 | 1.000 |

